

# Stress and Resistance Analysis for the Design of a Work Barge

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**Abstract**— A barge is usually a flat bottom vessel mainly used as cargo tanker, equipment supply carriers, crane platform and support accommodation bases in offshore operations. The work barge considered in this paper has special features like a deck crane, helicopter landing platform and a pollution department. This work barge serves a multipurpose offshore function for oil and gas industries, marine establishment and other companies that require manpower to work offshore without possible return of workers daily to shore. To ensure that this barge has the capacity to withstand all forms of stresses and forces that act on it, a good structural rigidity has to be ascertained. This work further estimated the weight of all components, machines, machineries, tanks, system etc to obtain the center of gravity which can be accomplished through a three dimensional analysis. With the help of the classification of ships' rules and regulations of Lloyd's Register various formulae were used to obtain various thickness of plates (side, bulk, deck etc), frame, girder, flanges, pillars etc. This helped to estimate the weight of the various component of the vessel. This includes all deck plates, frames, flanges, girder, pillars etc. The selection and positioning of these were in consonance with classification rules. Hence all forms of stresses were analyzed and the resistance of the vessel calculated. In all the analysis it was deduced that the work barge has the capacity to withstand all forms of stress and keep the vessel in a safe condition.

**Index Terms**— Stress analysis, Resistance analysis, Work barge, Plate thickness, Stiffness, Mass, Vessel.

## 1 INTRODUCTION

A Work barge is usually a flat bottom vessel mainly used as cargo tanker, equipment supply carriers, crane platform and support accommodation bases in offshore drilling. Most work barges do not operate under their own power but require a tugboat to pull or push them to their destination. Only in few cases do we see self-propelled work barges. Therefore barges are specially designed for specific purposes, depending on the type of barges, which is characterized by the function of the said barge, its design procedures are slightly different or rather the chosen characteristic may differ in one way or the other [1].

The work barge under consideration will have special features like a deck crane, helicopter landing platform and a pollution department. She will serve a multipurpose offshore function for oil and gas industries, marine establishment and other companies that require manpower to work offshore without possible return of workers daily to shore. To ensure that this barge has the capacity to withstand all forms of stresses and forces that will act on it, a good structural rigidity must be attained.

Therefore obtaining a good structural rigidity and estimation of the weight of all components, machines, machineries, tanks,

system etc to ascertain the center of gravity which can be accomplished through the three dimensional analysis<sup>7</sup> fronted by John in his work. With the help of the classification of ships' rule and regulation part 2 [2], part 3 [3], No 3 [4] and part 4 [5] of Lloyd's Register various formulas were used to obtain various thickness of plates (side, bulk, deck etc), frame, girder, flanges, pillars etc. hence to estimates the weight of the various component of the ship, that is, all deck plates (inner bottom plates, outer bottom plate, deck plate, 1<sup>st</sup> floor deck plate, 2<sup>nd</sup> floor deck plate and the plate for the helicopter platform), frames flanges, girder, pillars etc. The selection and positioning of these were in consonance with the iterative of Robert [6].

The strength calculation of the shear force and the bending moment determination and estimate were reviewed from Edward [7]. The estimations for tank selection were done from Nitonye [8] and Ekpenyong [9] from which insight came for the first stage of the estimations of tank capacity for the water, fuel, ballast etc. The final stage of the work will involve the analysis of the stability of the entire barge when deck crane is in offshore working condition, to ensure that the limit of load to be carried by the crane is not exceeded or points loads does not exceed a safe value for the vessel. There have been several development or giant strides made in the field of marine engineering in general and the design of barge in particular.

The design of a work barge of this magnitude with a crane and helicopter- landing platform following rules and regulation will yield several results from classification societies, laws, principles, experiments, calculations and assumptions

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etc. hence the structure must be strong enough to carry all induced stress (internal and external). Similarly, the structural arrangement of the designed work barge will be taken into consideration in this paper.

## 2 MATERIALS AND METHODS

### 2.1 Strength Calculations and Analysis

The calculations of the strength of the work barge are to enable us to know its ability to withstand the stress (es) or load(s) imposed on it while in operation. By this we will know and provide adequate strength without the structure(s) of the work barge yielding under normal condition of loading and even emergency situation [10]

Deck Plating Calculations

$$t = (6.5 + 0.02L)C \sqrt{\frac{KS_1}{S_b}} \quad (1)$$

$t$  = thickness of plating in mm

$L$  = length 80m

$$C = \frac{D + 2/3 - T}{\text{Height Of Deck Above Load Line At F.P.}} \quad (2)$$

By

$$C = \frac{6 + 2.3 - 4.5}{1.5} = 2.53$$

$$S_b = \text{Standard frame spacing} = \left[ 470 + \frac{L}{0.6} \right] \quad (3)$$

For forward of 0.05L from F.P.

By substituting, we have

$$S_b = \left[ 470 + \frac{80}{0.6} \right] = 603\text{mm}$$

Hence standard frame spacing 600mm is selected

$S_1 = S$  Spacing of secondary stiffness in mm = 600mm

$K = 0.66$  (from table)

Therefore by substituting into Equation 1

$$t = (6.5 + 0.02 \times 80)2.53 \sqrt{\frac{0.66 \times 600}{600}} = 16.65\text{mm}$$

Hence selected thickness ( $t$ ) for deck plating for his nature of barge is 17mm.

The given parameters

$$t = 17\text{mm}$$

$$L = 80\text{m}$$

$$B = 30\text{m}$$

Number of plate = 1

Chosen density of steel = 8.5 tonnes/m<sup>3</sup>

$$\text{Mass} = t \times L \times B \times \rho \times \text{Plate Number} \quad (4)$$

By substituting

$$\text{Mass} = 0.017 \times 80 \times 30 \times 8.5 \times 1 = 346.8\text{tonnes}$$

### 2.2 Double Bottom Plate Calculations

The depth of the double bottom is given by the formula (in millimeters)

$$d_{DB} = 32B + 190\sqrt{d} \text{ mm} \quad (5)$$

Where  $d$  = molded draft

$B$  = breadth of the vessel

By substituting

$$d_{DB} = 32 \times 30 + 190\sqrt{45} = 1363.1\text{mm}$$

While the center girder thickness is given by the relation

$$t = (0.008 d_{DB} + 4)\sqrt{k} \text{ mm} \quad (6)$$

by substitution

$$t = (0.008 \times 1363.1 + 4)\sqrt{0.66} = 12.11\text{mm}$$

For transverse frame thickness is given by the relation

$$t = (0.008 d_{DB} + 1)\sqrt{k} \text{ mm}$$

by substitution

$$t = (0.008 \times 1363.1 + 1)\sqrt{0.66} = 9.67\text{mm}$$

For longitudinal frame thickness is given by the relation

$$t = (0.0075 \times d_{DB} + 1)\sqrt{k} \text{ mm}$$

$$t = (0.0075 \times 1363.1 + 1)\sqrt{0.66} = 9.12\text{mm}$$

Selected  $t = 10\text{mm}$

For the double-bottom plate of this capacity of work barge, from Lloyds rule and regulation fore the classification of ships from part 4, chapter 1, section 7 and 8. The inner bottom plate thickness has the relation

$$t = 0.00136 (S + 660)^4 \sqrt{x^2 LT} \text{ (mm)} \quad (7)$$

$$t = 0.00136 (600 + 660)^4 \sqrt{0.66^2 \times 80 \times 4.5} = 6.06\text{mm}$$

Selected thickness  $t$  for inner bottom plate

$$t = 10\text{mm}$$

Therefore, given parameters

$$t = 10\text{mm}$$

$$L = 78\text{m}$$

$$B = 30\text{m}$$

$$P \text{ plate number} = 1$$

$$\text{Steel density} = 7.89\text{tonnes/m}^3$$

$$\text{Mass} = t \times L \times B \times \rho \times \text{Plate Number}$$

By substitution

$$\text{Mass} = 0.01 \times 78 \times 30 \times 7.89 \times 1 = 184.63\text{tonnes}$$

Hence, from previous selection, for the outer bottom plate our selected  $t = 15\text{mm}$

Therefore, given parameters

$$t = 15\text{mm}$$

$$L = 78\text{m}$$

$$B = 30\text{m}$$

$$\text{Plate number} = 1$$

Steel density = 8.5 tonnes/m<sup>3</sup>

**Mass = t x L x B x ρ x Plate Number**

By substitution

$$\text{Mass} = 0.015 \times 78 \times 30 \times 8.5 \times 1 = 298.3 \text{ 5tonnes}$$

### 2.3 Side Plate Calculations

From part 3 chapters 6, section 3 [4], the plate thickness is given by the relation

$$t = (6.5 + 0.03L) \sqrt{\frac{KS_1}{S_b}} \quad (8)$$

The parameter given

$$\begin{aligned} t &= ? \\ S_1 S &= 600\text{mm} \\ K &= 0.66 \\ L &= 80\text{m} \end{aligned}$$

Therefore by substitution into Equation 1, we have

$$t = (6.5 + 0.033 \times 80) \sqrt{\frac{0.66 \times 600}{600}} = 7.43\text{mm}$$

Selected thickness for side plate t = 10mm

**Mass = t x L x B x ρ x Plate Number**

$$\begin{aligned} \text{Steel plate thickness (t)} &= 10\text{mm} \\ \text{Length (L)} &= 80\text{mm} \\ \text{Height (D)} &= 6\text{rn} \\ \text{Number of plate} &= 2 \end{aligned}$$

$$\text{Mass} = 0.01 \times 6 \times 80 \times 7.89 \times 2 = 75.74 \text{ tonnes}$$

Aft Side Plate

$$\begin{aligned} \text{Steel plate thickness (t)} &= 10\text{mm} \\ \text{Breadth (B)} &= 30\text{rn} \\ \text{Height (D)} &= 6\text{m} \\ \text{Number of Plate} &= 1 \end{aligned}$$

$$\text{Mass} = 0.01 \times 6 \times 30 \times 7.89 \times 1 = 14.20 \text{ tonnes}$$

Fore Side Plate 1

$$\begin{aligned} \text{Steel thickness (t)} &= 10\text{mm} \\ \text{Breadth (B)} &= 30\text{m} \\ \text{Height (D)} &= 1.5\text{m} \\ \text{Number of plate} &= 1 \end{aligned}$$

$$\text{Mass} = 0.01 \times 1.5 \times 30 \times 7.89 \times 1 = 3.55 \text{ tonnes}$$

Fore Side Plate 2

$$L^2 = 4.5^2 + 5^2$$

$$L = 6.73 \text{ m}$$

Steel plate thickness (t) = 10mm

$$\begin{aligned} \text{Breadth (B)} &= 30\text{m} \\ \text{Height (D)} &= 6.73\text{m} \\ \text{Number of plate} &= 1 \\ \text{Density of steel} &= 7.89\text{tonnes/m}^3 \end{aligned}$$

$$\text{Mass} = 0.01 \times 6.73 \times 30 \times 7.89 \times 1 = 15.93 \text{ tonnes}$$

$$\text{Mass} = 0.01 \times 1.5 \times 30 \times 7.89 \times 1 = 3.55 \text{ tonnes}$$

### 2.4 Bulkheads Calculations

From part 4, chapter 1 section 9 [11] and [12] give the bulkhead thickness by this relation;

$$t = 0.004 S f \sqrt{h_A K} \quad (\text{mm}) \quad (9)$$

$$f = 1.1 - \frac{S}{2500s}$$

$$h_A = \text{Tank head} = 6\text{m}$$

$$S = \text{Space of member} = 15\text{m}$$

$$f = 1.1 - \frac{600}{2500 \times 15} = 1.084$$

Selected f = 1.0

Substituting these values into Equation 1

$$t = 0.004 \times 600 \times 1 \sqrt{6 \times 0.66} = 4.78$$

Selected thickness for longitudinal bulkhead is t = 8mm

Given parameters for

**Mass = t x L x B x ρ x Plate Number**

$$\begin{aligned} \text{Steel plate thickness (t)} &= 8\text{mm} \\ \text{Height} &= 6\text{rn} \\ \text{Length} &= 80\text{m} \\ \text{Density of steel} &= 7.89\text{tonnes/m}^3 \\ \text{Number of plate} &= 2 \end{aligned}$$

$$\text{Mass} = 0.08 \times 6 \times 80 \times 7.89 \times 2 = 60.6 \text{ tonnes}$$

For Transverse Stiffeners

$$t = 0.004 S f \sqrt{h_A K} \quad (\text{mm}) \quad (10)$$

$$f = 1.1 - \frac{600}{2500 \times 12.5} = 1.081$$

Selected f = 1.0

Substitute gives values into Equation 2

$$t = 0.004 \times 600 \times 1 \sqrt{6 \times 0.66} = 4.78$$

Selected thickness for transverse bulkhead is t = 8mm Given parameters for

**Mass = t x L x B x ρ x Plate Number**

$$\begin{aligned} \text{Steel plate thickness (t)} &= 8\text{mm} \\ \text{Height} &= 6\text{m} \\ \text{Breadth} &= 30\text{m} \\ \text{Density of steel} &= 7.89\text{tonnes/m}^3 \\ \text{Number of steel} &= 6 \end{aligned}$$

$$\text{Mass} = 0.08 \times 6 \times 30 \times 7.89 \times 1 = 68.17 \text{ tonnes}$$

### 2.5 Stiffness Calculations

From part 4, chapter 6, section 4 [10], we have it that longitudinal stiffness for deck;

Longitudinal Stiffness for Deck

$$\begin{aligned} \text{Breadth} &= 30\text{m} \\ \text{Number of stiffness} &= 50 \\ \text{Section } 152 \times 102 \times 8 &= 15.35\text{Kg/m} \end{aligned}$$

$$\text{Mass} = 80 \times 15.35 \times 50 \times = 61.4 \text{ tonnes}$$

Transverse Stiffness for Deck

Transverse space	=	600
Length of transverse	=	30m
Number of stiffness	=	134
Section	=	15.35Kg/m

$$\text{Mass} = 30 \times 15.35 \times 134 = 61.7 \text{ tonnes}$$

Longitudinal stiffness on side plate		
Height	=	6m
Number of longitudinal stiffness	=	10
Length of stiffness	=	80
Section 152 x 102 x 8	=	15.35Kg/m
Number of plate	=	2

$$\text{Mass} = 80 \times 15.35 \times 10 \times 2 = 24.6 \text{ tonnes}$$

#### Transverse Stiffness on Side Plate

Number of transverse stiffness	=	134
Length of transverse	=	6m
Length of stiffness	=	6m
Section = 152x102x8	=	15.35Kg/m
Number of plate	=	2

$$\text{Mass} = 6 \times 15.35 \times 134 \times 2 = 24.7 \text{ tonnes}$$

### 2.6 Longitudinal Stiffness on Bottom Plate (Double)

Breadth	=	30m
Length of longitudinal	=	75m
Number of bottom plate	=	2

$$\text{Mass} = 75 \times 15.35 \times 50 \times 2 = 115.1 \text{ tonnes}$$

#### Transverse Stiffness of Bottom Plate (Double)

Breadth	=	30m
Length of transverse	=	30m
Number of transverse	=	134
Number of plate	=	2

$$\text{Mass} = 30 \times 15.35 \times 134 \times 2 = 123.4 \text{ tonnes}$$

#### Longitudinal Stiffness on Bulkhead

Number of longitudinal	=	10
Length of longitudinal	=	80m
Number of bulkheads	=	2

$$\text{Mass} = 10 \times 15.35 \times 80 \times 2 = 24.6 \text{ tonnes}$$

#### Transverse Stiffness on Bulkhead

Number of transverse	=	134
Length of transverse	=	6m
Number of bulkhead	=	2

$$\text{Mass} = 6 \times 15.35 \times 134 \times 2 = 24.7 \text{ tonnes}$$

#### Transverse Web Frame Side to Side

Total number of frames	=	11
Total length of web with flange	=	92.8mm
Flange	=	80.28mm
Total transverse web frame length	=	172.56mm

$$\text{Mass} = 172.56 \times 0.01 \times 450 \times 8.5 \times 11 = 72 \text{ tonnes}$$

### 2.7 First Floor Deck (Tier) Superstructure Calculations

For Deck Plating From part 3, Chapter 5 and section 1 and 2  
 For  $L \leq 100m$  is given by the relations

$$t = 5.5 + 0.02L \sqrt{\frac{KS}{S_b}} \quad (11)$$

By substitution <sup>(9)</sup>

$$t = (5.5 + 0.02 \times 50) \sqrt{\frac{0.66 \times 600}{600}} = 5.28mm$$

Selected thickness for deck plating first floor is  $t = 8mm$  given parameters

t	=	8mm
Length	=	47m
Breadth	=	27m
Plate	=	1
Density of steel	=	7.89tonnes

$$\text{Mass} = 0.008 \times 47 \times 27 \times 7.89 = 80.1 \text{ tonnes}$$

Side Plating for 1<sup>st</sup> Tier

From Lloyd's

$$t = [5.0 + 0.01L_3] \sqrt{K} \quad (12)$$

For  $L_3 = 49m$

$$K = 0.66$$

By substituting

$$t = [5.0 + 0.01 \times 49] \sqrt{0.66} = 4.5$$

Selected thickness for side plate for first tier floor is  $t = 6.0mm$  similarly to obtain the height of the superstructure for first floor from Lloyd's section 1.4.2

$$h = 2.5 + 0.01L_2 \quad (13)$$

$$L_2 = 49m$$

$$h = 2.5 + 0.01 \times 49 = 2.99m$$

Selected height  $h = 3.0m$

Giving parameters to calculate the mass

t	=	6m
Length	=	49
Height	=	3m
Number of plate	=	2

$$\text{Mass} = 0.006 \times 47 \times 3 \times 2 \times 7.89 = 13.35 \text{ tonnes}$$

Fore and Aft Side Plate, given parameters

t	=	6.0mm
Breadth	=	27m
Height	=	3m
Number of plate	=	2

$$\text{Mass} = 0.006 \times 27 \times 3 \times 2 \times 7.89 = 7.67 \text{ tonnes}$$

### 2.8 Bulkheads for First Tier

From Lloyd's Part 3 Chapter 8, Section 2 Thickness of Bulkhead

$$t = 0.003S \sqrt{Kh} \quad (14)$$

By substituting

$$t = 0.003 \times 600 \sqrt{0.66 \times 3} = 2.53$$

Selected thickness for bulkhead (longitudinal)

$$t = 5\text{mm}$$

Given parameters

$t$	=	5mm
Height	=	3m
Length of bulkhead	=	132m
Density of steel	=	7.89tonnes/m <sup>3</sup>

$$\text{Mass} = 0.005 \times 3 \times 132 \times 7.89 = 15.6 \text{ tonnes}$$

Selected thickness for bulkhead (transverse)

$t$	=	5mm
Height	=	3m
Length of bulkhead	=	237m
Density	=	7.89tonnes/m <sup>3</sup>

$$\text{Mass} = 0.005 \times 3 \times 237 \times 7.89 = 28.05 \text{ tonnes}$$

## 2.9 Stiffness for First Tier

Longitudinal Stiffness for Deck

Longitudinal spacing	=	600mm
Breadth	=	27m
Length	=	47m
Number of longitudinal stiffness	=	45
Sections	=	7.89 tonnes/m <sup>3</sup>

$$\text{Mass} = 47 \times 45 \times 7.89 = 16.7 \text{ tonnes}$$

Transverse Stiffness for Deck

Transverse spacing	=	600mm
Breadth	=	27m
Number of stiffness	=	78
Section	=	7.89 tonnes/rn <sup>3</sup>

$$\text{Mass} = 27 \times 7.89 \times 78 = 16.6 \text{ tonnes}$$

Longitudinal Stiffness of Side Plates

Height	=	3m
Number of longitudinal	=	5
Length	=	47
Number of plates	=	2

$$\text{Mass} = 5 \times 47 \times 2 \times 7.89 = 3.71 \text{ tonnes}$$

Transverse Stiffness of Side Plates

Height	=	3m
Number of transverse	=	78
Number of plates	=	2

$$\text{Mass} = 3 \times 78 \times 2 \times 7.89 = 3.70 \text{ tonnes}$$

Longitudinal Bulkhead Stiffness (Longitudinal)

Height	=	3m
Number of longitudinal	=	5
Length	=	132
Density of Steel	=	7.89 tonnes/m <sup>3</sup>
Number of plates	=	1

$$\text{Mass} = 5 \times 132 \times 1 \times 7.89 = 5.21 \text{ tonnes}$$

Height	=	3m
Number of transverse	=	220

$$\text{Density of steel} = 7.89 \text{ tonnes/m}^3$$

$$\text{Number of plates} = 1$$

Transverse Bulkheads Stiffness (Longitudinal)

Height	=	3m
Number of transverse	=	237
Density of steel	=	7.89 tonnes/m <sup>3</sup>
Number of plates	=	1

$$\text{Mass} = 5 \times 237 \times 7.89 = 9.4 \text{ tonnes}$$

Transverse Bulkheads Stiffness (transverse)

Height	=	3m
Number of transverse	=	395
Density of steel	=	7.89 tonnes/m <sup>3</sup>
Number of plates	=	1

$$\text{Mass} = 3 \times 132 \times 1 \times 7.89 = 9.4 \text{ tonnes}$$

## 2.10 Frames

Longitudinal frames thickness	=	10mm
Total frame	=	6
Total length	=	50m
Number of plate	=	2

$$\text{Mass} = 50 \times 0.01 \times 6 \times 7.89 \times 2 \times 450 = 21.3 \text{ tonnes}$$

Transverse frame thickness	=	10mm
Total frame	=	3
Total length	=	28mm
Number of plate	=	2

$$\text{Mass} = 28 \times 0.01 \times 3 \times 7.89 \times 2 \times 450 = 5.96 \text{ tonnes}$$

Second Floor (Tier) Superstructure

For Deck Plating

From part 3, chapter 8, section 2<sup>(9)</sup> for  $L \leq 100\text{m}$  is given by the relation;

$$t = 5.0 + 0.02L_3 \sqrt{\frac{KS}{S_b}} \quad (15)$$

By substitution

$$t = (5.0 + 0.02 \times 49) \sqrt{\frac{0.66 \times 600}{600}} = 4.83\text{mm}$$

Selected thickness for deck plating second floor is  $t = 6\text{mm}$

Given parameters

$t$	=	6mm
Length	=	40m
Breadth	=	28m
Plate	=	1
Density of steel	=	7.89 tonnes/m <sup>3</sup>

$$\text{Mass} = 0.006 \times 40 \times 28 \times 7.89 \times 1 = 53.02 \text{ tonnes}$$

Side Plating for Second Floor

From Lloyd's

$$t = [4.0 + 0.01L_3] \sqrt{K} \quad (16)$$

By substitution

$$t = [4.0 + 0.01 \times 49] \sqrt{0.66} = 4.4\text{mm}$$

Selected side plate thickness  $t = 5\text{mm}$

While the height of the second floor is given by the relation

$$h = 1.25 + 0.005L_2$$

by substituting

$$h = 1.25 + 0.005 \times 49 = 1.5 \text{ m}$$

Selected height for second floor is 2.5m

Giving parameters to obtain the mass of side plate as:

$t$	=	5m
$Length$	=	49
$Height$	=	2.5m
$Number\ of\ plate$	=	2

$$Mass = 0.005 \times 49 \times 7.89 \times 2 \times 2.5 = 9.7 \text{ tonnes}$$

Fore and Aft Side Plate

$t$	=	5.0mm
$Breadth$	=	28m
$Height$	=	2.5m
$Number\ of\ plate$	=	2
$Density$	=	7.89 tonnes/m <sup>3</sup>

$$Mass = 0.005 \times 28 \times 7.89 \times 2 \times 2.5 = 5.52 \text{ tonnes}$$

### 2.11 Helicopter Landing Area

From Part 3, Chapter 9, Section 3 and 5 [4]. The deck plate thickness  $t$ , within the landing area is given by;

$$t = t_1 + 1.5 \text{ (mm)} \quad (17)$$

$$\text{where } t_1 = \frac{\alpha S}{1000\sqrt{K}} \quad (18)$$

$$\text{and } \beta = \log \left( \frac{\rho_1 K^2}{S^2} \times 10^7 \right) \quad (19)$$

The plating is to be designed for the emergency landing; case taking

$$P_1 = 2.5\phi_1\phi_2\phi_3 f\gamma P_w \text{ tonnes} \quad (20)$$

From tables

$n$	=	Type correction factor	=	1.0
$P_w$	=	load on the type print	=	6tonnes
$P_1$	=	corrected patch load	=	?
$\lambda$	=	Dynamic configuration factor	=	1.7
$\phi_1$	=	patch aspect ratio correction factor	=	1.0
$\phi_2$	=	Panel aspect ratio correction factor	=	1.0
$\phi_3$	=	wide patch load factor	=	1.0
$f$	=	landing decks over marred spaces	=	1.15
$\gamma$	=	location factor	=	0.6

Substituting the following parameters into Equation 20, gives

$$P_1 = 2.5 \times 1 \times 1 \times 1 \times 1.15 \times 0.6 \times 6 = 10.35 \text{ tonnes}$$

Therefore,  $\beta$  from Equation

$$\beta = \log_{10} \left( \frac{10.35 \times 0.66^2}{600^2} \times 10^7 \right) = \log_{10} 125.235 = 2.10$$

From the Tyre print chart

When  $\beta = 2.1$  and  $v/s \geq 1.0$

$$\alpha = 8$$

Substituting into Equation 2

$$\therefore t_1 = \frac{8 \times 600}{1000\sqrt{0.66}} = 5.91\text{m}$$

$$\therefore t = 5.91 + 1.5 = 7.41$$

Selected thickness for helicopter landing area thickness is

$$t = 20\text{mm}$$

### Third Floor (Tier) Superstructure For deck plating,

Given parameters

$t$	=	20mm
$Length$	=	20m
$Breadth$	=	20m
$Plate\ No.$	=	1
$Density\ of\ steel$	=	7.89 tonnes/m <sup>3</sup>
$Mass = 0.002 \times 20 \times 7.89 \times 20 \times 1$	=	63.1 2tonnes

### 2.12 Stiffness for Third floor

Longitudinal Stiffness for Deck

$Longitudinal\ spacing$	=	600mm
$Length$	=	20m
$Breadth$	=	20m
$Number\ of\ longitudinal\ members$	=	33
$Section$	=	7.89 tonnes/m <sup>3</sup>

$$Mass = 20 \times 7.89 \times 33 = 5.21 \text{ tonnes}$$

Transverse Stiffness for Deck

$Transverse\ spacing$	=	600mm
$Breadth$	=	20m
$No.\ of\ transverse$	=	33
$Section$	=	7.89 tonnes/m <sup>3</sup>

$$Mass = 20 \times 7.89 \times 33 = 5.21 \text{ tonnes}$$

The shear force diagram gives a representation of the upward and downward forces acting on the work barge as shown in figure 1

### 2.13 Shear and Bending Moment

A work barge of overall length of 80m and length between perpendiculars as 75m has the breadth of 30m, depth of 6.0m and a maximum loading draft as 4.5m. The work barge is assumed to be statically stable on a sinusoidal wave, in which the height of the wave at any point about the still water wave is given as

$$H = 0.607\sqrt{L} = 5.43\text{m}$$

$$L = \text{Overall Length} \quad (21)$$

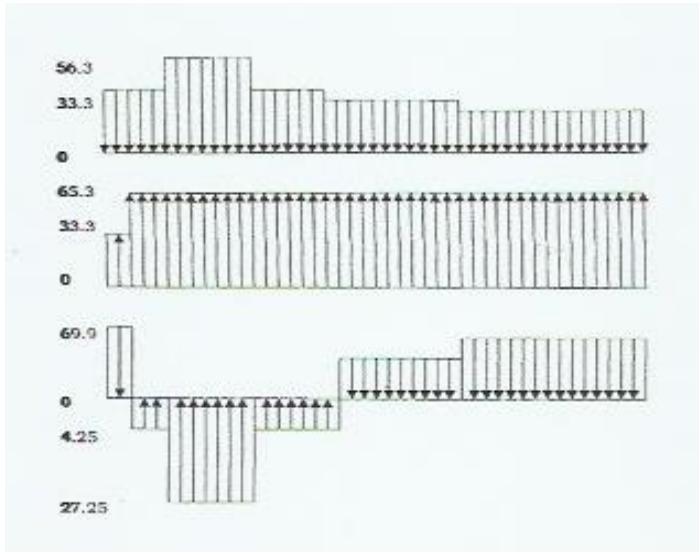


Figure 1: Shear Force Diagram of the Barge.

## 2.14 Wave Load Distribution

At a quarter length

$$\begin{aligned}
 \text{Buoyancy moment} &= 45,937.50 \\
 \text{Mass moment} &= 36,250 \text{ tonnes/m} \\
 \text{Net Moment} &= \text{Buoyancy} - \text{Mass Moment} \\
 &= 9,687.50 \text{ tonnes/m}
 \end{aligned}$$

To convert to Nm, we will multiply by "g", that is 9.81m/s<sup>2</sup>

$$\text{Still water bending moment} = 95,034.375 \text{ Nm} = 95.024 \text{ MN}$$

When we consider wave as a cosine form and will have the crests at the ends as

$$H = \frac{5.43}{2} \cos \frac{2\pi x}{80} \quad (22)$$

$$\begin{aligned}
 \therefore \text{Buoyancy per unit meter} \\
 = \rho b g h \div 1000 = 0.819 \cos \frac{2\pi x}{80} \text{ MNm} \quad (23)
 \end{aligned}$$

We know that the integration of the force due to the Buoyancy gives us the shear force due to the wave.

$$= \int 0.819 \cos \frac{2\pi x}{80} dx \quad (24)$$

$$\frac{0.819}{2\pi} \times 80 \sin \frac{2\pi x}{80} + A$$

$$\text{at } x = 0; A = 0$$

$$x = 30 \text{ m}$$

$$\text{Wave shear force} = 0.429 \text{ MN}$$

On the other hand, integrating the wave shear force gives the wave bending moment

$$\text{Therefore, Wave Bending Moment} = \int \frac{0.819 \times 80}{2\pi} \sin \frac{2\pi x}{80} dx$$

The condition that the bending moment is zero

at  $x = 0$  gives

$$B = \frac{0.819 \times 80^2}{2\pi^2}$$

At  $x = 40 \text{ m}$

The wave bending moment

$$\frac{0.819 \times 80^2}{2\pi^2} \left[ 1 + \cos \frac{2\pi \times 40}{80} \right] = 265.3 \text{ MNm} \quad (25)$$

Therefore, the total Bending Moment at sagging condition

$$= \text{Still water moment} + \text{Wave moment} = 360.334 \text{ MNm}$$

## 2.15 Sectional Modulus

### Properties of Mild Steel

$$\text{Ultimate tensile strength} = 400 - 495 \text{ MN/m}^2$$

$$= 26 - 32 \text{ tonnes/m}^2$$

$$\text{Yield stress} = 230 - 250 \text{ MN/m}^2 = 15 - 16 \text{ Tonnes/m}^2$$

$$\text{Shearing strength} = 22 \text{ tonnes/m}^2$$

We know that

$$1. \text{ Stress} = \frac{\text{Force (Load)}}{\text{Unit Area (m}^2\text{)}} \quad (26)$$

$$2. \text{ Factor Of Safety} = \frac{\text{Yield Stress}}{\text{Max Design Stress}} \quad (27)$$

From these equations we derive that

$$\frac{\text{Factor}}{\text{Unit Area}} = \frac{\text{Yield Stress}}{\text{Factor of Safety}} \quad (28)$$

$$\therefore \text{Unit Area} = \frac{\text{Force} \times \text{Factor of Safety}}{\text{Yield Stress}} \quad (29)$$

Simple beam theory

- $M_B$  = Sino
- $M_B$  = Bending moment
- $S_M$  = Sectional Modulus
- $\sigma$  = Unit Stress

$$\sigma = \frac{M_B}{S_M} \tag{30}$$

$$\sigma = \frac{M_B \times C}{I} \tag{31}$$

Where C is the distance from the neutral axis (a line parallel to the base line from the Centroid of all the effective longitudinal strength members comprising the section)

I = Sectional moment of inertia about the Natural axis

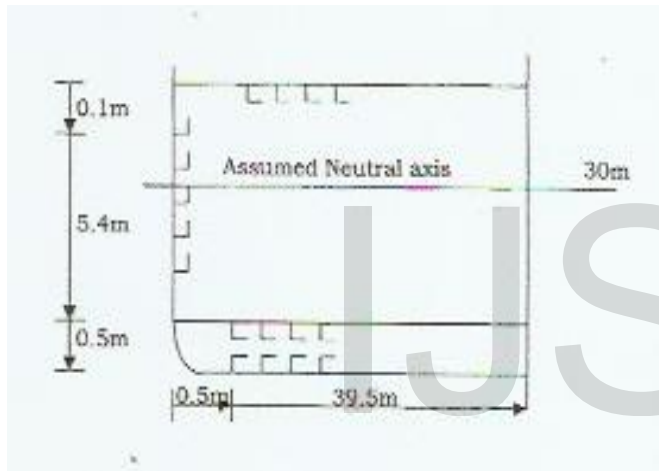


Figure 2: Barge Section

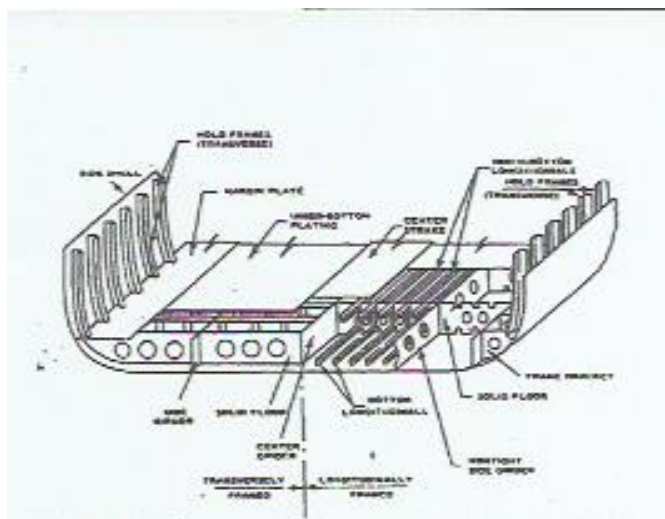


Figure 3: Structural element of the barge hull [13]

The barge section gives us an internal representation of the

barge and how the stiffness is placed at different sections of the barge as shown in figure 2 and figure 3 shows the structural element of the work barge hull. And the table below shows the calculation of the sections, which helped in the force analysis of the work barge.

$$\text{Height of Neutral Axis } h_{NA} = \frac{\sum ah}{\sum a} \tag{32}$$

$$= 2.14\text{m above the keel}$$

$$\text{Second moment of area of half section above base} \\ = \sum ah^2 + I_o = 3.3138\text{m}^4 \tag{33}$$

$$\Sigma \text{Parallel axis term} = \Sigma a \times h^2_{NA} = 2.046\text{m}^4 \tag{34}$$

$$I_{NA} = \text{Second moment of area of half section about the base - parallel axis term} \tag{35}$$

$$= 1.2678\text{m}^4$$

$$\text{Therefore; } I_{NA} (\text{Full section}) = 2.5356\text{m}^4$$

$$\text{Full area} = 0.89364\text{m}^2$$

$$Z\text{-Deck} = \frac{I_{NA} \text{ Full}}{\text{ShipHeight} - I_{NA}} = 0.6569\text{m}^3 = \tag{36}$$

$$Z\text{-Deck} = \frac{I_{NA} \text{ Full}}{\text{ShipHeight} - I_{NA}} = 0.6569\text{m}^3 = \tag{37}$$

$$Z\text{-Base} = \frac{I_{NA} \text{ Full}}{h_{NA}} = 1.1849\text{m}^4 \tag{38}$$

$$\text{Factor of Safety} = 3$$

Maximum design stress

$$\frac{\text{Factor}}{\text{UnitArea}} = \frac{\text{YieldStress}}{\text{Factor of Safety}} = 83.33\text{MN} / \text{m}^2 \tag{39}$$

Using the maximum bending moment included in the steel structure

$$\text{Stress on Deck} = \frac{M_B}{Z_P} = 548.54\text{MN} / \text{m}^2$$

$$\text{Stress on base} = \frac{M_B}{Z_B} = 304.1\text{MN} / \text{m}^2$$

**2.16 Resistance Calculations and Analysis**

To estimate the total resistance of this tonnage of work barge, it is commended to split this resistance into two components

1. The frictional resistance ( $R_P$ ) basing on Reynolds number -  $R_n$



2. The residuary resistance ( $R_R$ ) basing on Froud number  $F_r$  (that is, the wave-making resistance and Eddy Resistance).

$d$  = draft of the ship  
 $\nabla$  = Volume of displacement  
 $C_B$  = Block coefficient  
 $B$  = Beam

$$\therefore \text{Total Resistance } R_T = R_F + R_B \quad (40)$$

$$R_F = C_F \frac{\rho v^2 s}{2} \quad (41)$$

$$R_R = C_R \frac{\rho v^2 s}{2} \quad (42)$$

$C_T, C_F, C_R$  are the total frictional and residuary resistance coefficient respectively. The frictional resistance is the resistance due to the motion of the hull through a viscous fluid. It is as high as 80-85% of the total resistance in slow - speed vessel and as much as 50% in high-speed vessel. The value of friction divers in laminar flows and that of turbulent flows is shown below. Hence low at laminar flow with Reynolds Number [14]

$$R_e = \frac{VL}{V} \quad (43)$$

In 1904 Blasins

$$C_F = \frac{R_F}{0.5 \rho s v^2} \quad (44)$$

$$1.327 \left[ \frac{VL}{V} \right]^{-\frac{1}{2}} \quad (45)$$

For laminar flow

While in 1921 Prandtl and Von Karma published the equation

$$C_F = \frac{R_F}{0.5 \rho s v^2} \quad (46)$$

$$0.072 \left[ \frac{VL}{V} \right]^{-\frac{1}{5}} \quad (47)$$

For turbulent flow

$$R_F = f s v^n \quad (48)$$

$R_F$  = Resistance  
 $s$  = total area surface (wetted surface)  
 $v$  = speed (knots)

The wetted surface area  $s$  may be estimated using the Mumford formula

$$S = 1.7 L_{PP} \times d + \frac{\nabla}{d} \quad (49)$$

or Bruckhoffe's formula

$$S = \frac{(4d + B) \times \frac{L}{2}}{1.625 - C_B} \quad (16) \quad (50)$$

$L_{PP}$  = Length of the ship between perpendicular (ft)

### 2.17 The Air Resistance ( $R_A$ )

This resistance is calculated from the experiment of Admiral Taylor's empirical formula

$$R_A = 0.004 \times 0.5 B^2 \times V_R^2 \quad (51)$$

Where  $B$  = Beam of the ship  
 $V_R$  = Relative velocity of the wind  
 $V_V$  =  $V$  (speed of the ship) in still air

$$R_A = C_A \times 0.5 \rho A_T \times V^2 \quad (52)$$

Where  $C_A$  = Resistance coefficient  
 $\rho$  = Mass density of air  
 $A_T$  = Transverse project area of above water hull  
 $V$  = Ship speed  
 $A_T = 0.3A_1 + A_2$  (53)

Where  $A_1$  = Main hull area  
 $A_2$  = super structure area

Others supposed to be estimated but considered negligible include

- Wake - making resistance ( $R_w$ )
- Eddy - making resistance ( $R_e$ )

Note: because the work barge is not self-propelled it will not have appendage resistance. From Guildhammer-Harold method of resistance calculation, usable values include

$L_{PP}$  = 75.0m  
 $B$  = 30m  
 $T$  = 4.5m  
 $\nabla$  = 4900 tonnes  
 $\frac{L}{\nabla^{1/2}}$  = 4.53  
 $V$  = 6.43m/s  
 $\rho$  = 10.25kg/m<sup>3</sup>  
 $\nu$  = 0.9425 x 10<sup>-6</sup> m<sup>2</sup>/s Kinematics viscosity coefficient at

25°C seawater  
 $C_p$  = 0.993  
 $\therefore \frac{V}{\sqrt{L}} = \frac{6.43}{\sqrt{75.0}} = 0.742$

Reynolds number  $R_n = \frac{VL}{\nu} = 5.12 \times 10^8$

Coefficient of Frictional Resistance  $C_F$

$$C_F = C_F + dC_F \quad (54)$$

From I.T.T.C.

$$\begin{aligned} R_n &= 5.0 \times 10^8 \\ C_{FO} &= 1.671 \times 10^{-3} \\ R_n &= 5.5 \times 10^8 \\ C_{FO} &= 1.651 \times 10^{-3} \end{aligned}$$

For  $R_n = 5.12 \times 10^8$

By interpolation

$$\frac{0.12}{0.50} \times 0.02 = 0.0048$$

$$\therefore C_{FO} = (1.6662 \times 10^{-3}) \approx 1.67 \times 10^{-3}$$

But  $C_{FO} =$

$$= \frac{R_F}{\frac{1}{2} \rho V^2 S} \quad (55)$$

$$= \frac{0.075}{[\log R_n - 2]^2}$$

$$dC_F = 0.4 \times 10^{-3} \text{ for } L \leq 90m$$

$$C_F = 2.07 \times 10^{-3}$$

### 2.18 Coefficient of Residuary Resistance ( $C_{R0}$ )

From graph of CR, Vs,  $\frac{\sqrt{V}}{L}$  [15]

$$\frac{L}{\sqrt{V}} = 4.53$$

$$\frac{V}{\sqrt{L}} = 0.742$$

Correction for (B/T)-  $\nabla C_{RI}$

$$\nabla C_{RI} = 0.12 \left[ \frac{B}{T} - 2.07 \right] \times 10^{-7}$$

By substituting

$$\nabla C_{RI} = 0.552 \times 10^{-3}$$

Correction for Length of centre of buoyancy  $F_{CB}$ -  $\nabla C_{R2}$

$$\text{For } \frac{V}{\sqrt{L}} \leq 0.6, \quad C_{R2} = 0$$

$$\text{For } \frac{V}{\sqrt{L}} < 0.6, \quad (56)$$

$$C_{R2} = a \left[ \frac{V}{\sqrt{L}} - 0.6 \right] \Delta L_{PP}$$

Where a = 0.75 for  $C_p = 0.993$

### 3.9.1 Air and Wind Resistance

$$R_{AA} = C_{AD} \times \frac{1}{2} \rho A_T V^2 \quad (57)$$

Admiral Taylor, from experiment in air derived a resistance coefficient of 1.28.

$$R_{AA} = 1.28 \times \frac{1}{2} \rho A_T V^2$$

$V_R$  = Apparent wind velocity or wind velocity relative to the ship In still air,  $V_R = V$

$$\rho \text{ of air} = 0.00238$$

Area of air Resistance  $A_T = 0.3A_1 + A_2 = 2.235m^2$

In still water  $R_{AA} = 14.08$

Total Resistance  $C_T = C_F + C_R = 3.35 \times 10^{-3}$

Assuming  $C_T$  due to other resistances not taken into consideration, let

$$C_T = 3.35 \times 10^{-3}$$

$$R_T = C_T = \frac{\rho V^2}{2} S$$

$$S = C \sqrt{\Delta L_{WL}} \quad (58)$$

(Taylor's formulas)

but  $C = 2.6$

$$L_{NL} = L_{PP} + 3.5\% \text{ of } L_{PP} = 77.625m$$

$$\therefore S = 2.6 \sqrt{4900 \times 77.625} = 1603.512m^2$$

$$\therefore R_T = C_T \frac{\rho V^2}{2} S$$

$$= \frac{5.35 \times 10^{-3} \times 1025 \times 6.43^2 \times 1603.5}{2} = 181.8KN$$

## 3 RESULTS AND DISCUSSIONS

In anticipation of the barge floating in an upright condition at many different water lines (or draft) in the course of its services, it is usual to calculate, in advance, the main geometrical characteristics of the ship form. This data below shows the masses of steel structures and various loads on the barge

1. Mass of the steel structure	=	1,400 tonnes
2. Mass of first floor and loads	=	200 tonnes
3. Mass of second floor and loads	=	230 tonnes
4. Mass of third floor	=	100 tonnes
5. Mass of Helicopter	=	50 tonnes
6. Mass of crane load	=	300 tonnes
7. Mass of machinery load	=	70 tonnes
8. Mass of tanks loads	=	1,200 tonnes
9. Mass of cargo loads	=	1,000 tonnes
10. Mass of other loads	=	300 tonnes

Evenly distributed load along the entire length of the work

barge

\* Weight of steel + weight of tanks = 1,400 + 1,200  
 = 2,600 tonnes

Therefore;  $\frac{2600}{80} = 32.5 \text{ tonnes/m}$

- \*Weight at Helideck point = 7.5 tonnes/m
- \*Weight at first floor point = 4.0 tonnes/m
- \* Weight at second floor point = 5.75 tonnes/m
- \* Weight at crane point = 30.0 tonnes/m
- \* Weight at machinery point = 1.75 tonnes/m
- \* Weight at cargo point = 33.33 tonnes/m
- \* Weight at others = 3.75 tonnes/m
- Buoyancy force = 65.33 tonnes/m

**Table 1 Scantling/Section Calculation of the barge**

Items	Scantlings	Area (A) m <sup>2</sup>	Height (h) m	Moment (ah) m <sup>3</sup>	2 <sup>nd</sup> moment (ah <sup>2</sup> ) m <sup>4</sup>	Local 2 <sup>nd</sup> moment (I <sub>o</sub> ) m <sup>4</sup>
Strength deck	5.4x17mm	0.0918	0.2	0.01836	0.00367	
Longitudinal stiffness	102 x 102 x 7.8	0.00224	2.35	0.0054	0.0129	
Side	5.4x10	0.054	0.2	0.0108	0.00216	6.1728
plating Longitudinal	102 x 102 x 7.8	0.00224	2.39	0.0054	0.0129	
Side stiffness	102 x 102 x 7.8	0.009224	14.39	0.0326	0.4134	
Bottom plating	29.4x10mm	0.294	3	0.882	2.646	
		0.44652		0.95416	3.141	0.1728

The weight of the work barge was estimated to be 3361.1 tonnes with all necessary equipments, machineries, machine, tanks, pumps, plates, etc. there by having the capacity of carrying external load up to 1000 tonnes within the vessel depending on the safety factor and the available space. After design and estimation, it was observed that the center of gravity is acting at 2.84m above the keel (bottom) of the barge, 1.39m fore of the chosen centre (longitudinally) and 19mm port of the chosen centre (transversely). This is safe in ship design; it would have been unsafe if the centre of gravity acts above 3.0m from the bottom of the ship.

Looking at the hydrostatic curves, the perfect straight line suggests that the draft varies directly with the displacement of the barge shows its ability to withstand all stresses and float upright. Figures 4 to 7 shows the stability characteristics of the work barge with respect to draft and other parameters like the centre of gravity, meta-centric height, displacement and distance between center of buoyancy and meta-center

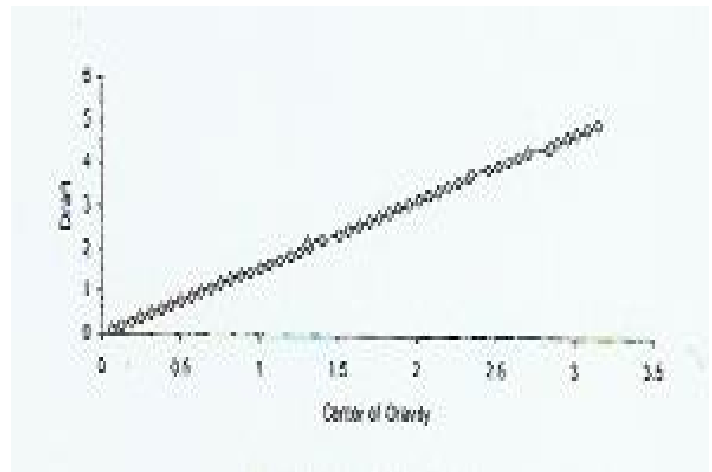


Figure 4: Draft Vs Centre of Gravity [1]

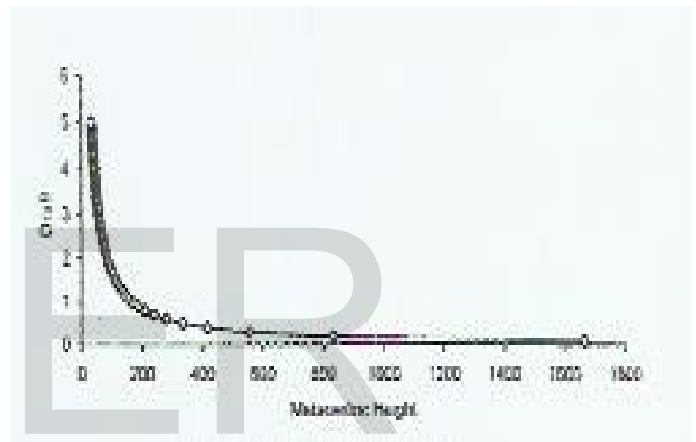


Figure 5: Draft Vs Metacentric Height

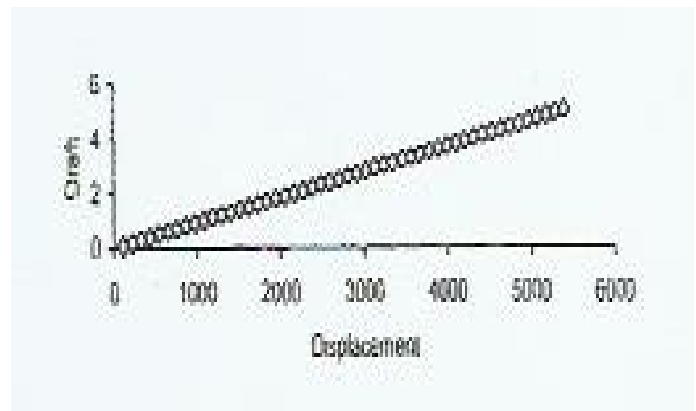


Figure 6: Draft Vs Displacement

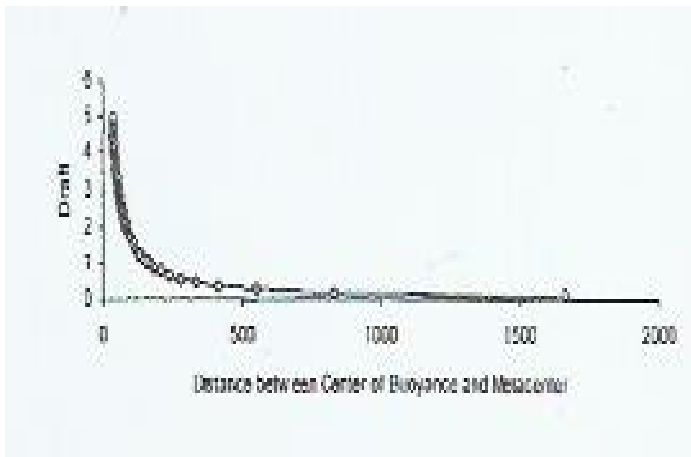


Figure 7: Draft Vs Distance between Centre of Bouyance and Metacenter

#### 4 CONCLUSION

The results of this research shows that the deck would be in a compression stress while the bottom would be on a tensile stress. The stress obtained is within the allowable stress hence the vessel structure would be able to withstand all forms of stresses that the barge will encounter during all offshore operations. Also the results of the longitudinal transverse vertical centre of gravity and meta-centric height showed that the barge would be stable at all normal condition of loading.

In order to obtain the moment of inertia giving a safe stress with maximum material, the materials are disposed further away from the neutral axis and most efficiently converted to have its designed share of the stress. Hence the (strength) stress which the structure needed is withstands compare with the maximum allowable yield stressed from Lloyd's handbook indicated that the barge structure would withstand the hogging and sagging and six motion of gyration that would undergo during the course of its service.

The analysis revealed that the hull form components weigh a total mass of 3361.1 ton and the moments along the X, Y and Z components were within the safe limit. This show that the barge can withstand a maximum stress of 83.33MN/m<sup>2</sup>, haven a bending moment induced on the deck at 548.54MN/m<sup>2</sup> maximum and on the base at 304.1MN/m<sup>2</sup> maximum. Appendix 1 shows the results of all design calculations for the work barge.

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## 5 APPENDIX 1

Table 3 Results of Design Calculations of the work barge

Components	Scantlings (mm)	Thickness (mm)	Quantity (No)	Unit Length (m)	Weight (Tonnes)
<b>A. HULL</b>					
1. Bottom plate (outer)	75x30x15	15	1	75	298.4
2. Bottom plate (inner)	75x30x10	10	1	75	184.6
3. Deck plate	80x30x17	17	1	80	346.8
4. Side plate	80x6x10	10	2	80	75.74
5. Force plate (rectangle)	30x1.5x10	10	1	30	3.55
6. Force plate (inclined)	30x6.73x10	10	1	30	15.93
7. Aft plate	30x6x10	10	1	30	14.20
8. Bottom Longitudinal stiffness	152x102x7.8		50	75	57.5
9. Deck longitudinal stiffness	152x102x8.0		50	80	61.4
10. Longitudinal bulkhead	3800x8	8	2	80	60.6
11. Transverse bulkhead	3800x8	8	8	80	88.04
12. Longitudinal bulkhead	152x89x8		20	80	24.6
13. Longitudinal bulkhead	152x89x8		264	6	24.288
14. Transverse bulkhead longitudinal stiffness)	152		20	15	4.6
15. "	"		60	30	27.6
16. "	"		30	7.5	3.6
17. Transverse bulkhead (transverse) stiffness	152x102x8		408	6	37.536
18. Side plate longitudinal stiffness	152x89x8		20	6	24.6
19. Side plate transverse stiffness	152x89x8		266	6	24.472
20. Bottom flange	508 x 102 x 10		21	Different length	15.52
21. Deck flange	451 x 102 x 8		21	Different length	11.28

Components	Scantlings (mm)	Thickness (mm)	Quantity (No)	Unit Length (m)	Weight (Tonnes)
22. Side flange	406 x 127 x 8		14	Dif. Length	6.08
23. Longitudinal bulkhead flange	406 x 102 x 8		14	Dif. length	5.68
24. Transverse bulkhead flange	406 x 102 x 8		36	96	10.28
25. Bottom angle stiffness	152 x 108 x 8		6	80	7.2
26. Top angle stiffness	152 x 108 x 8		6	80	7.2
27. Longitudinal pillars	152 x 152 x 10		12	6	1.728
28. Transverse deck beam	152 x 152 x 10		6	30	4.32
29. Transverse web moment			24	3.4	2.4
30. Girder	152 x 152 x 10		3	80	5.76
31. Outer bottom longitudinal	152 x 102 x 8		50	80	31.0
32. Inner bottom longitudinal	152 x 102 x 6		50	80	31.0
33. Outer boom Transverse	152 x 102 x 6		266	30	61.18
<b>B. SUPERSTRUCTURE (1<sup>st</sup> FLOOR)</b>					
1. Deck plate	47 x 23 x 8	8	1	47	80.1
2. Force plate	27 x 3 x 6	6	1	27	3.83
3. Aft plate	27 x 3 x 6	6	2	27	3.84
4. Side plate	47 x 3 x 6	6	2	47	12.07
1. Side plate longitudinal stiffness starboard port	152 x 89 x 6		6	47	2.22
	152 x 89 x 6		6	38	1.89
6. Side plate transverse stiffness	152 x 89 x 6		141	3	2.82
7. Deck longitudinal stiffness	152 x 89 x 6		45	47	16.59
8. Deck transverse stiffness	152 x 89 x 6		79	27	16.59
9. Longitudinal bulkhead	152 x 89 x 6	6	14	Dif. length	3.19

Components	Scantlings (mm)	Thickness (mm)	Quantity (No)	Unit Length (m)	Weight (Tonnes)
10. Transverse bulkheads	152 x 89 x 6	6	15	Different length	4.10
11. Side flange	406 x 127 x 8		12	Different length	2.75
12. Longitudinal bulkhead flange	406 x 127 x 8		19	Different length	4.93
13. Transverse bulkhead flange	406 x 127 x 8		21	Different length	6.11
14. Deck angle stiffness	152x 108 x 8	8	2	47	1.42
15. Top angle stiffness	152x 108 x 8	8	2	47	1.42
16. Longitudinal pillars	152x 152 x 10	10	12	3	0.864
17. Transverse Deck beam	152x 152 x10	10	6	27	3.90
18. Transverse web			24	3.4	2.40
19. Angle web			120	3.4	0.6
<b>C. SUPPER STRUCTURE (2<sup>ND</sup> FLOOR)</b>					
1. Deck plate	40 x 28 x 6	6	1	40	53.02
2. Fore plate	28 x 2.5 x 5	5	1	28	2.76
3. Aft plate	28 x 2.5 x 5	5	1	25	2.76
4. Side plate (star board)	49 x 2.5 x 5	5	1	49	4.85
5. Side plate (Port)	40 x 2.5.5	5	1	40	3.95
6. Side plate longitudinal stiffness (starboard)	152 x 89 x 6		5	49	1.95
7. Side plate longitudinal stiffness (port)	152 x 89 x 6		5	40	1.60
8. Side plate transverse stiffness	152 x 89 x 6		164	2.5	3.28
9. Deck longitudinal stiffness	152 x 89 x 6		47	40	18.33
10. Deck transverse stiffness	152 x 89 x 6		164	28	36.08
11. Longitudinal bulkhead	40 x 2.5 x 5	5	6	40	23.7
12. Transverse bulkhead	7 x 2.5 x 5	5	28	7	16.52

Components	Scantlings (mm)	Thickness (mm)	Quantity (No)	Unit Length (m)	Weight (Tonnes)
13. Side flange	406 x 127 x 8		8	10.3	2.8
14. Longitudinal bulkhead flange	406 x 102 x 8		24	10.3	8.4
15. Transverse bulkhead flange	406 x 102 x 8		28	6.5	5.88
16. Deck angle stiffness	152x 108 x 8		4	40	2.4
17. Top angle stiffness	152x 105 x 8		4	40	2.4
18. Longitudinal pillars	152x 152 x 10		20	2.5	1.2
19. Transverse Deck beam	152x 152 x10		5	28	3.35
20. Transverse web			20		2.0
21. Angle web			250		1.28
<b>D. SUPPER STRUCTURE (3<sup>RD</sup> FLOOR)</b>					
1. Deck Plate	40 x 20 x 20	20	1	20	63.12
2. Deck Longitudinal stiffness	152 x 102 x 8		33	20	5.28
3. Deck transverse stiffness	152 x 102 x 8		33	20	5.28
4. Side flange	406 x 127 x 510		16	5.6	3.04
5. Deck angle stiffness	152 x 108 x 8		4	20	1.20
6. Longitudinal pillar	152 x 153 x 10		20	2.5	2.0
7. Transverse Deck beam	152 x 153 x 10		10	20	4.8
8. Transverse web			60		0.6
9. Longitudinal web			48		0.48
<b>F. Estimations of Machineries, Tanks and others</b>					
1. Crane	-	-	1	-	250
2. Helicopter	-	-	1	-	20
3. Generators	-	-	4	-	20
4. Purifiers	-	-	2	-	0.1
5. Ballast pumps	-	-	3	-	0.078
6. Service pumps	-	-	2	-	0.052
7. Fire/Bilge Pumps	-	-	3	-	0.078



Components	Scantlings (mm)	Thickness (mm)	Quantity (No)	Unit Length (m)	Weight (Tonnes)
8. Lathe Machine	-	-	1	-	1.36
9. Sawing Machine	-	-	1	-	0.2
10. Welding set	-	-	1	-	0.15
11. Press Unit	-	-	1	-	0.015
12. Tools and equipment	-	-	1	-	0.10
13. Fresh water transfer pumps	-	-	2	-	0.052
14. Sea water cooling pumps	-	-	2	-	0.05
15. Fresh Water cooling pumps	-	-	1	-	0.025
16. Sewage treatment plant	-	-	1	-	1.50
17. Water maker	-	-	1	-	4.0
18. Boilers	-	-	2	-	4.0
19. Laundry Machine	-	-	1	-	1.2
20. Air conditioner plant	-	-	1	-	2.0
21. Lube oil service pump	-	-	1	-	0.026
22. Refrigerators	-	-	2	-	0.03
23. Baking oven	-	-	1	-	0.02
24. Fresh water service pumps	-	-	2	-	0.05
25. Anchors	-	-	4	-	20.0
26. Fuel tanks	-	-	3	-	495.0
27. Fresh water tank	-	-	4	-	500.0
28. Hospital equipment	-	-	1	-	0.2
29. Canteen equipment	-	-	1	-	0.2
30. Toilet blocks	-	-	Many	-	0.7
31. Recreation room equipments	-	-	Many	-	0.15
32. Radio room equipment	-	-	Many	-	0.15
33. Office equipment	-	-	Many	-	0.45
34. Mass equipment	-	-	Many	-	0.30
35. Accommodation facilities	-	-	Many	-	0.30